

DESIGN OF RETAINING WALL

- Design Data
- 1 Height of Earth to be Retained
 - 2 Slope, α
 - 3 Coeff. Of Friction b/w Soil & Concrete, μ
 - 4 Gross Safe Bearing Capacity of Soil
 - 5 Unit Weight of soil, γ
 - 6 Depth of Foundation
 - 7 P.C.C Below Footing
 - 8 Surcharge Angle, β
 - 9 Earth Pressure Coeff., Active, K_a
 - 10 Earth Pressure Coeff., Passive, K_p
 - 11 Total Height of Retaining Wall, H

Material Used

- Concrete Grade
- Steel Grade
- Unit Weight of R.C.C

Section of Retaining Wall

- Thickness of Base Slab, BT
- Length of Base Slab, B
- Length of Heel Slab, L
- Stem Thickness, ST

Height of Wall Above Base Slab, h

- UNITS
- 6.00 m
 - 30 DEGREE
 - 0.50 RADIANS
 - 0.50
 - 175 kN/m³
 - 21 kN/m²
 - 0.80 m
 - 0.375 m
 - 30 DEGREE
 - 0.30
 - 3.0
 - 6.40 m
- M 25
Fe 420
- 23.5 kN/m³

Course	Soil
	Retained Water

DOF = (SBC/500 -
integrated number of 2

DOF = height of earth

Stability Checks

Table-2

S.No	Description	Force, KN		Dist. from Heel, m	Moment, kNm
1	Weight of Soil above Heel (W1)	W1 = $\gamma \times h \times B$	200.312	1.20	240.37
2	Weight of Stem (W2)	W2 = $\gamma \times B \times T$	27.540	2.70	74.26
3	Weight of Base Slab (W3)	W3 = $\gamma \times B \times L$	62.544	2.50	156.36
4	Surcharge	$\gamma \times H \times L \times \mu \times \sin \alpha$	153.032	2.70	413.60
	Total (W)		443.43		884.59

* Note: Passive Earth Pressure Not Considered.

Dis. Of Point of Application of Resultant Force from Heel and, Z

2.42 m

Eccentricity $e = Z - B/2$

-0.07 < $B/6 = 0.833$

Eccentricity less than $B/6$, so there is no uplifting, Hence Safe

Extreme Earth Pressure at the Base, $W/B(L \pm e \times K)$

Minimum Pressure

97.32 kN/m²

Pressure = $W/B \times (1 - e \times K)$

Maximum Pressure

82.68 kN/m²

Pressure = $W/B \times (1 + e \times K)$

Maximum Pressure less than SBC hence safe

Check for Overturn

Resisting Moment about Toe, M_r

1255.50 kNm

Overturning Moment about Toe, M_o

370.888 kNm

Factor of Safety against Overturning, $0.875 M_r/M_o$

2.8030712

Factor of Safety greater than 1.4 hence safe

condition FOS > 1.4, if not redesign

Check for Sliding

Resisting Force Due to Overlying Weight, μW

88768

236.49854 kN

Sliding Force, P_a (Active Pressure Force)

88768

553.83 kN

Coefficient of Sliding Friction, μ

88768

1.46

Factor of Safety greater than 1.4 hence safe

condition FOS > 1.4, if not redesign

	0
	0.00
Coarse Grained (silt)	0.40
silt	0.30
coarse Rock	0.6

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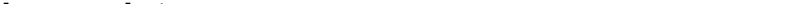
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